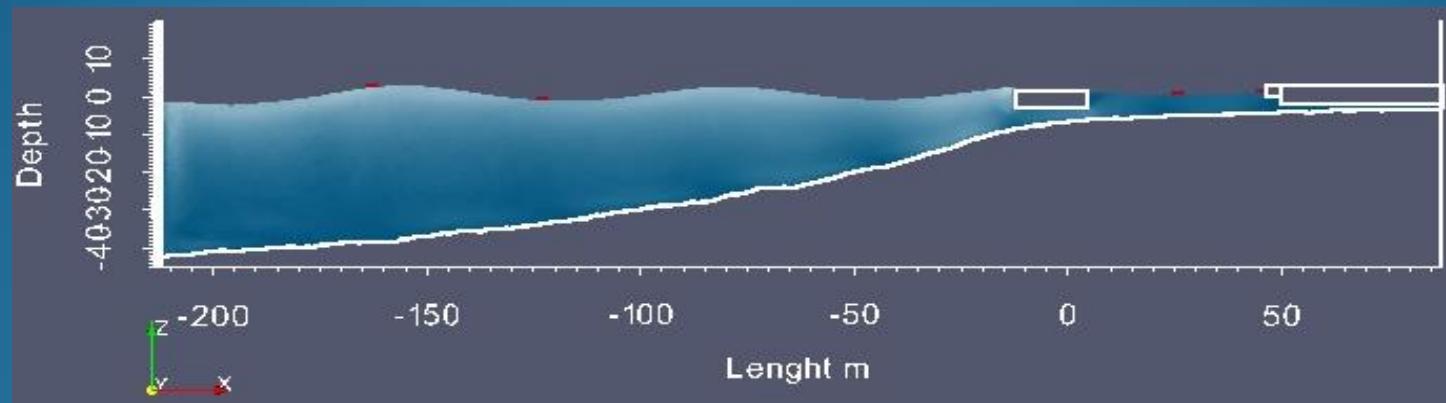


Performance Optimization of a Floating Breakwater Model Using SPH Method, with a Practical Application

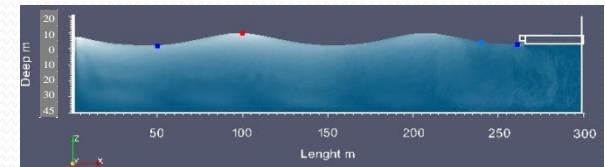


Directed By : PhD G. Rodríguez
PhD A. Crespo

Alejandro Rueda Duran
May 31 of 2013

Agenda

1. Problem Study Description
 - a. Survey Area
 - b. Floating Breakwaters (FB) Overview
2. Smoothed Particle Hydrodynamics - SPH
3. Methodology
 - a. Box Model Design
 - b. Floating Breakwater Study Case Design
4. Results and discussion
 - a. Box Model Validation
 - b. Santa Marta Case
5. Conclusions
6. Future work
7. References
8. Questions



1. Problem Study Description

a. Survey Area



1. Problem Study Description

a. Survey Area



- Marina
- Breakwater

Manzanares River

EGSAM Pier

1. Problem Study Description

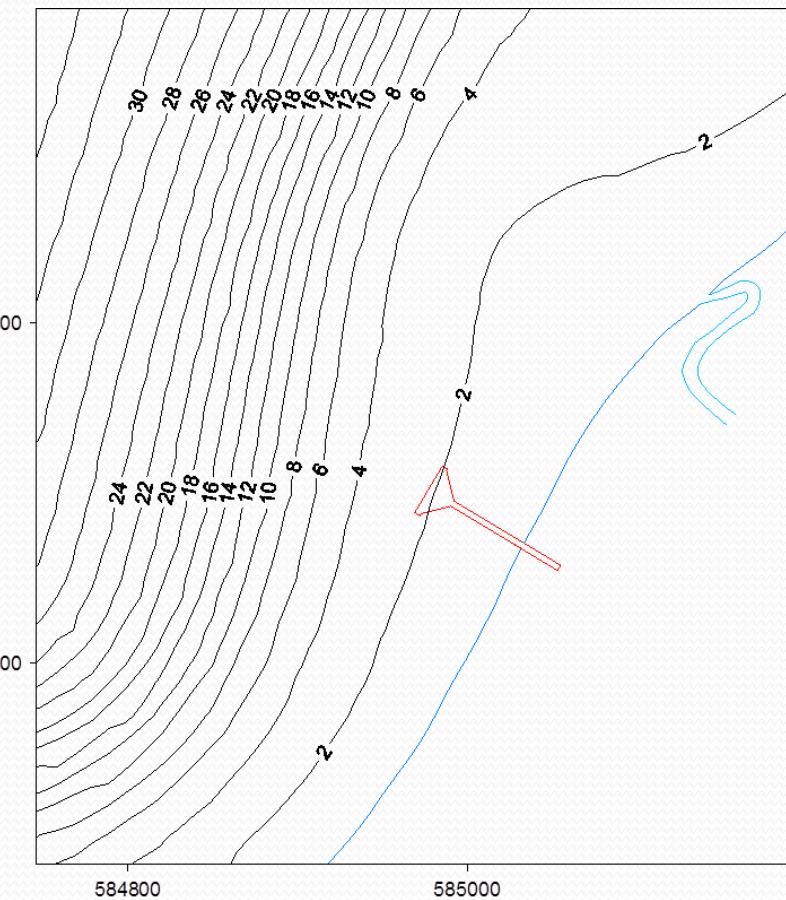
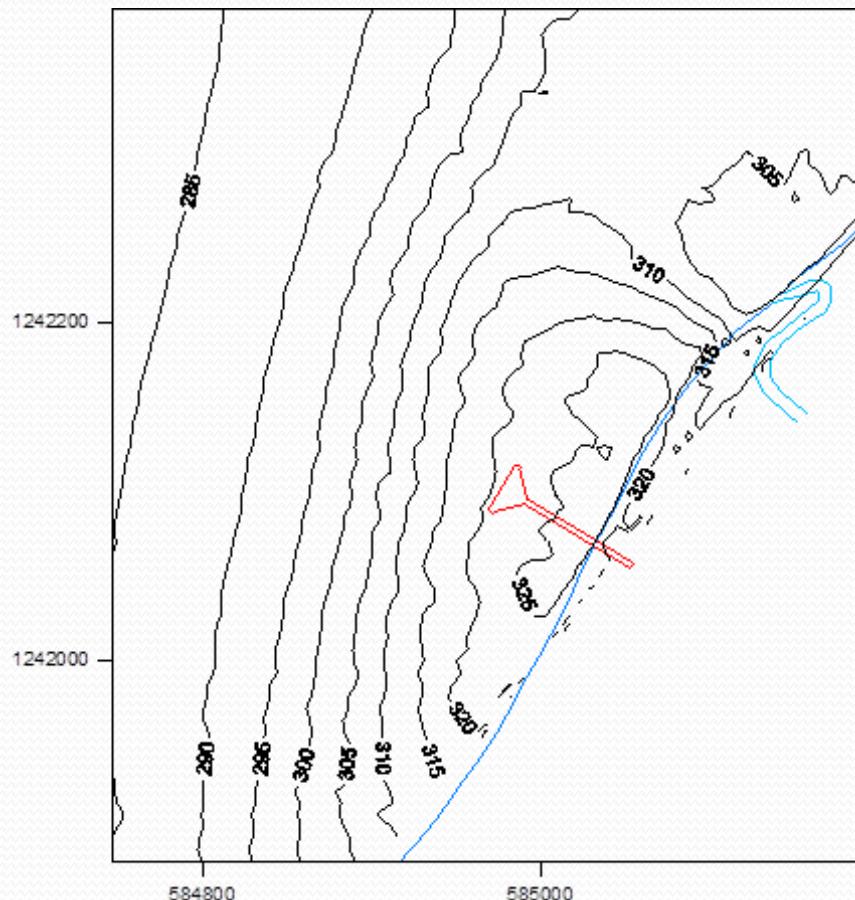
a. Survey Area



Images from: Google Earth

1. Problem Study Description

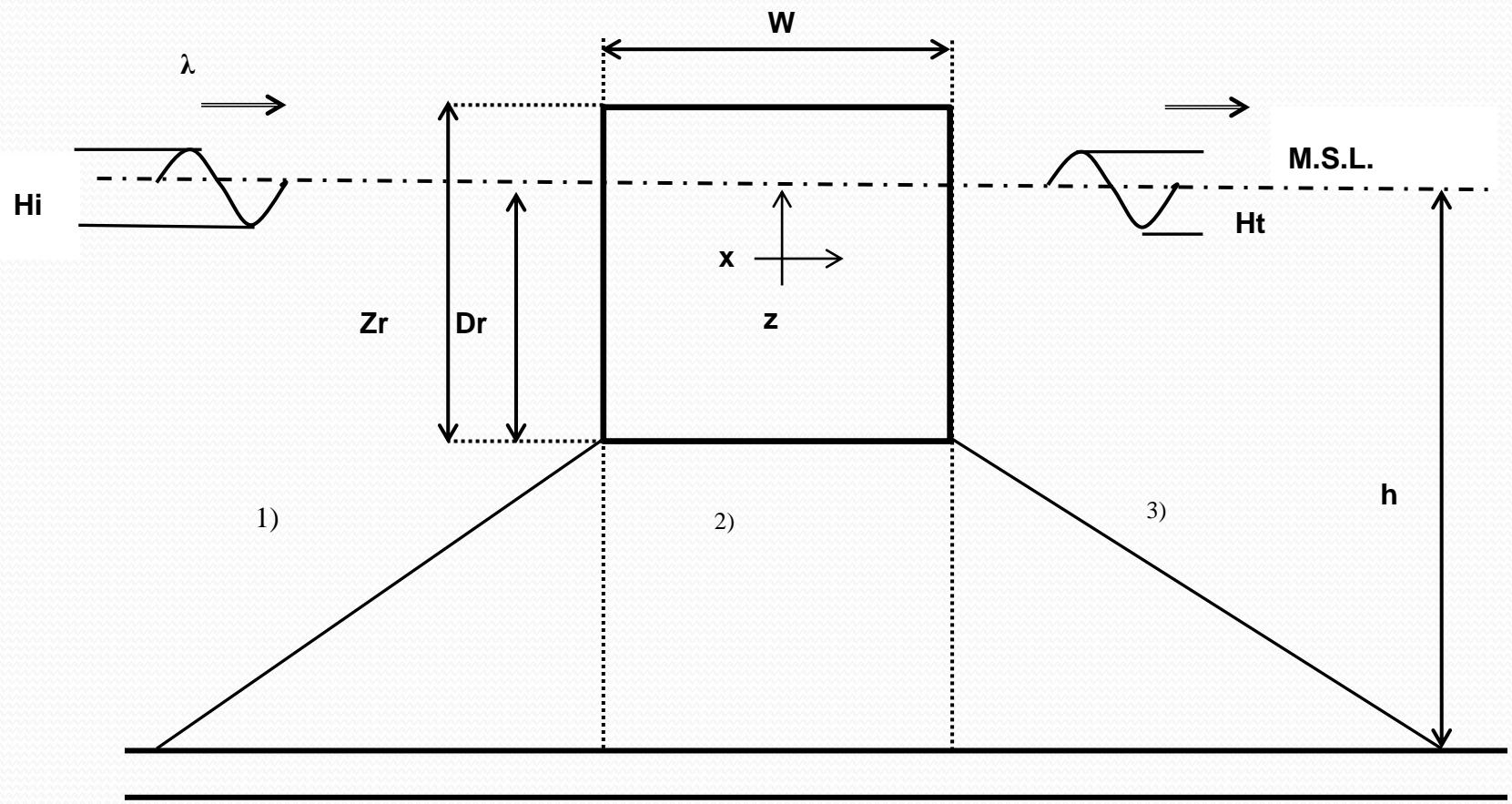
a. Survey Area



Images from: Sergei Lonin

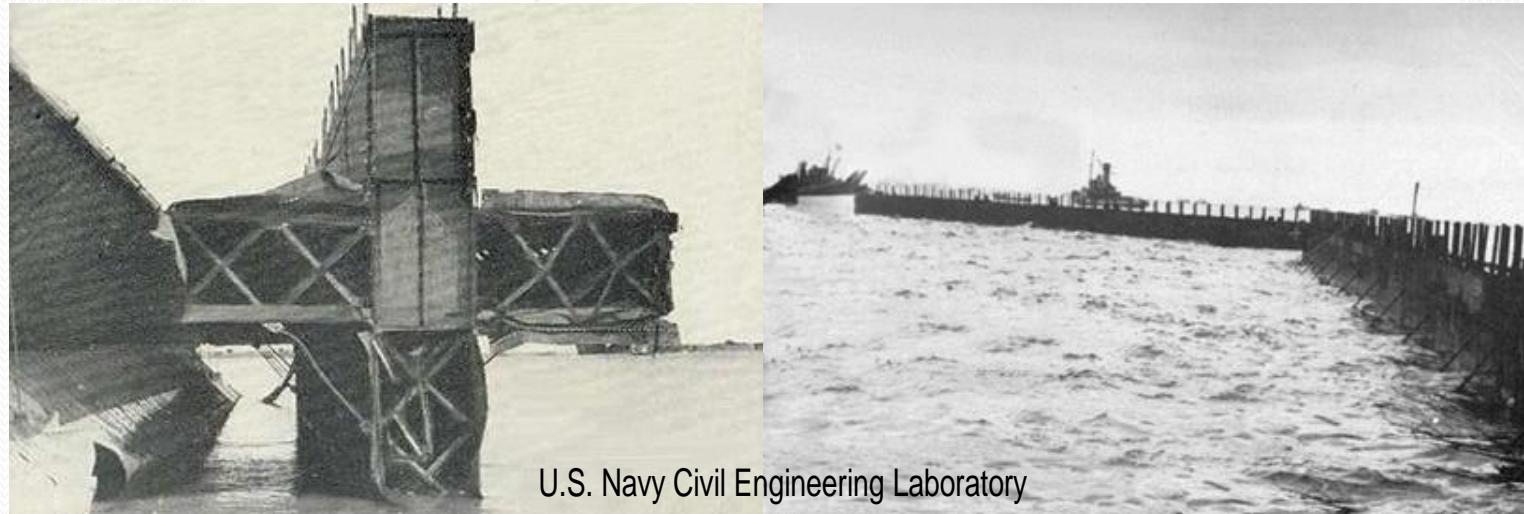
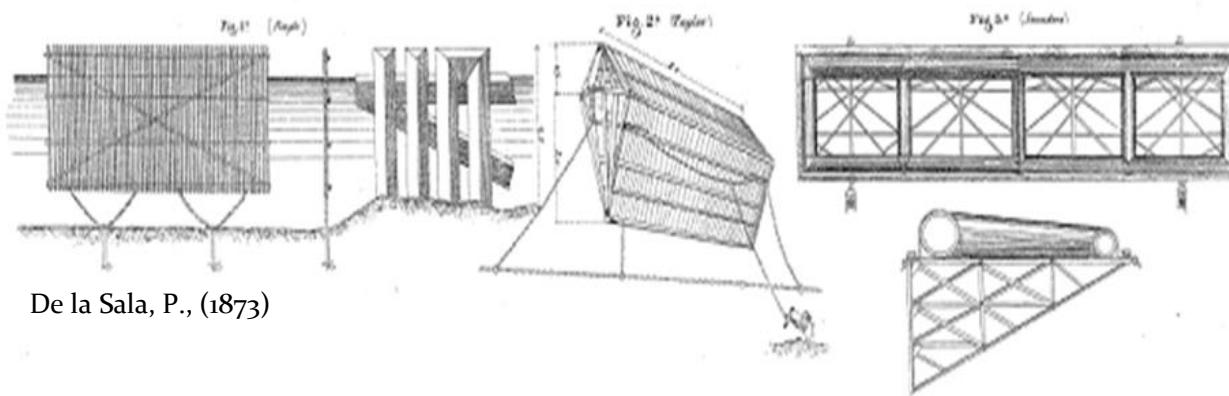
1. Problem Study Description

b. Floating Breakwaters Overview



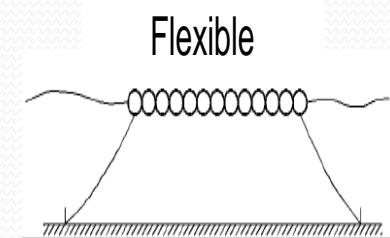
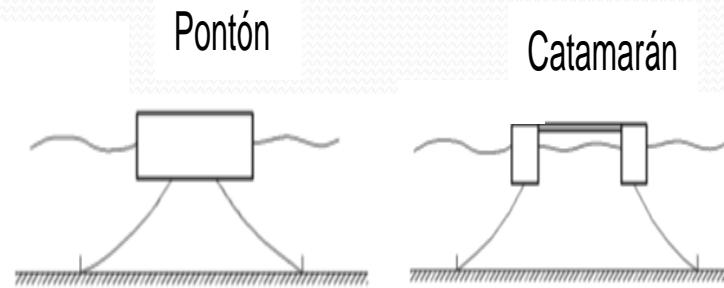
1. Problem Study Description

b. Floating Breakwaters Overview



1. Problem Study Description

b. Floating Breakwaters Overview

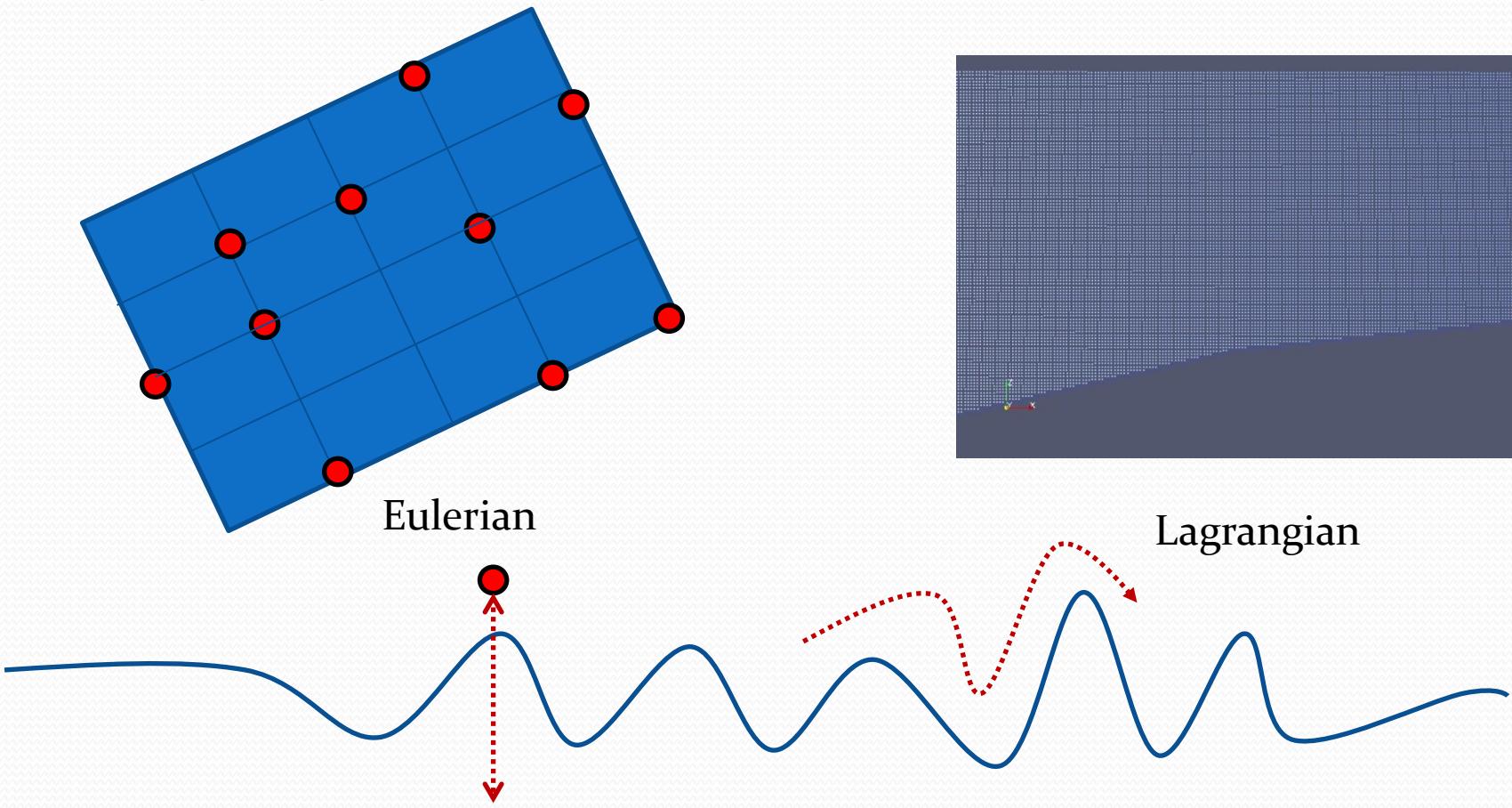


2. Smoothed Particle Hydrodynamics - SPH

- Astrophysics and Cosmology, Gingold and Monaghan (1977) and Lucy (1977)
- SPHysics (2007) y DualSPHysics (2010)
 - University of Vigo (Spain),
 - University of Manchester (UK) and
 - The Johns Hopkins University (U.S.A.)
- Free Code www.dual.sphysics.org
 - Fortran95
 - C++

2. Smoothed Particle Hydrodynamics - SPH

- Lagrangian Method



2. Smoothed Particle Hydrodynamics - SPH

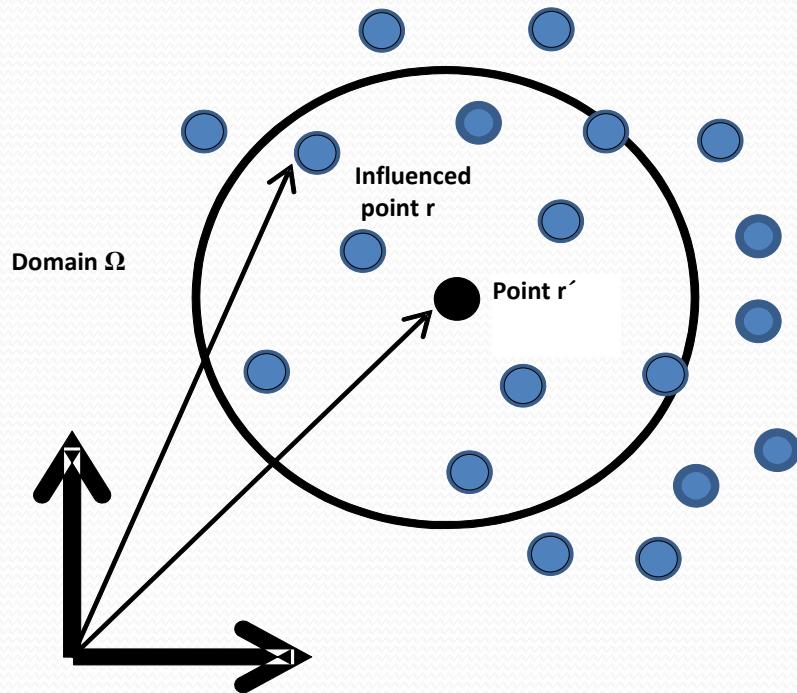
- Reality is reproduced (fluid) y (objects): Particles
 - Properties (e.g., mass, ρ , vel., position and P)
 - To represent the passing from a continuous medium (fluid) to a discrete (particles), SPH uses the kernel function

$$A(\mathbf{r}) = \int_{\Omega} A(\mathbf{r}') W(\mathbf{r} - \mathbf{r}', h) d\mathbf{r}' \quad (1)$$

$$W_{ab} = W(r_a - r_b, h) \quad (2)$$

- Where r is the particle position; W is the weighting function or kernel; h is the weighting function smoothing length controls the domain Ω

2. Smoothed Particle Hydrodynamics - SPH



1. Controls the interaction of each particle
2. Reduced computational cost - estimate only where fluid
3. It is slower to having to propagate "many" particles

2. Smoothed Particle Hydrodynamics - SPH

- Momentum Equation. Monaghan (1992)

$$\bullet \frac{d\mathbf{v}_a}{dt} = - \sum_b m_b \left(\frac{P_a}{\rho_a^2} + \frac{P_b}{\rho_b^2} \right) \nabla_a W_{ab} + \mathbf{g} \quad (3)$$

- Artificial Viscosity. Monaghan (1992)

$$\bullet \frac{d\mathbf{v}_a}{dt} = - \sum_b m_b \left(\frac{P_a}{\rho_a^2} + \frac{P_b}{\rho_b^2} \right) \nabla_a W_{ab} + \mathbf{g} \quad (4)$$

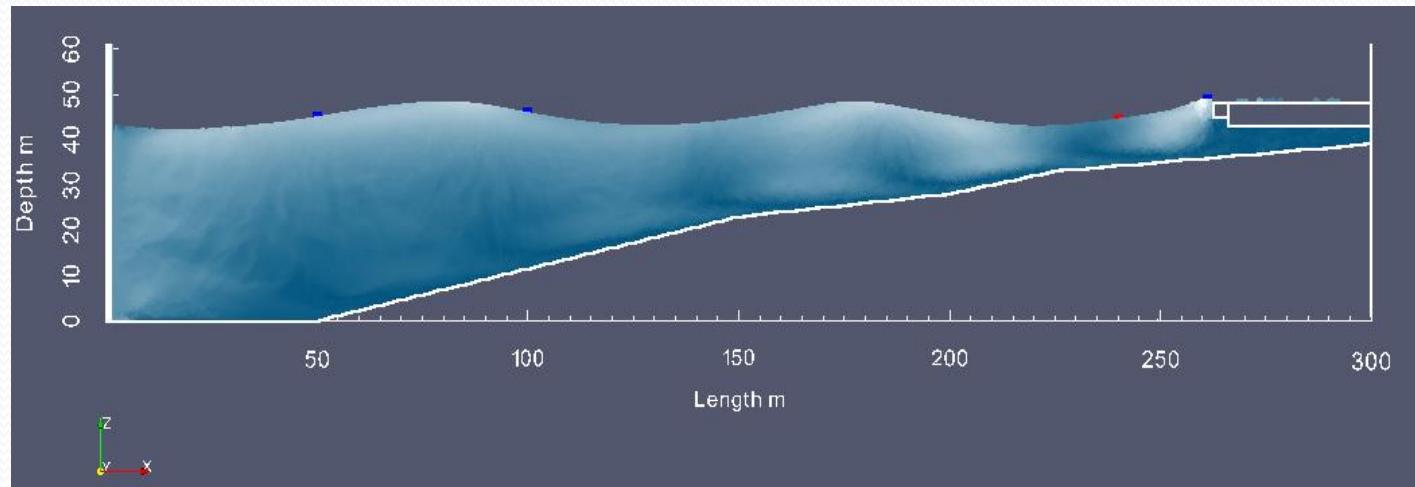
- *Continuity Equation*

$$\bullet \frac{d\rho_a}{dt} = \sum_b m_b \mathbf{v}_{ab} \nabla_a W_{ab} \quad (5)$$

1. Methodology

a. Box Model Design

The following dimensions were used in the box model design:



- Separation between particles: 0.25m.
- 121,812 particles were used in 2D and 7,264,747 in 3D.
- A bathymetry designed to strokes

3. Methodology

b. FB Study Case Design

- To validate the model we used the structure proposed by Bruce (1985) for Friday Bay (Washington).
- For FB design a methodology was employed by the authors using a relationship :
 - Wide (w)
 - Height (Z_R)
 - Draft (D_r)
 - Depth (h)
 - Wave length(λ)
 - Period (T)

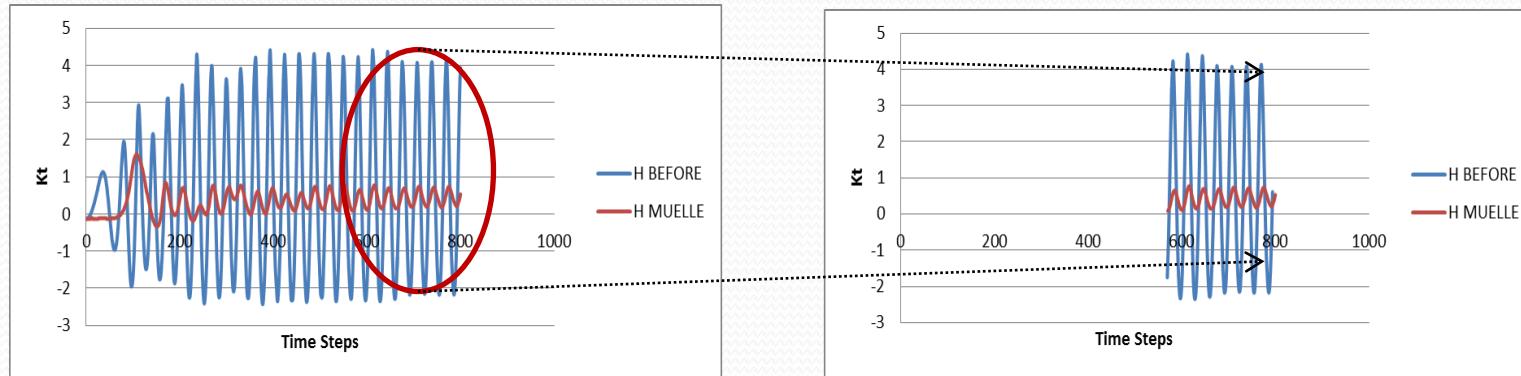
3. Methodology

a. FB Study Case Design

References	Thesis Code	Place	Height FB /Draft Zr/Dr	Wide / Draft W/Dr	Wave Height/ Depth H/h	Depth/ Wave Length h/λ	Wide/ Wave Length W/λ	Draft / Depth Dr/h	Wave Height/ Wave Length Hi/λ	Dimentional Depth $2\pi h/\lambda$
Brebner (1968)	Case 1	Physical Model	2,221844	7,5647092	0,200	0,345	0,383	0,147	0,069	2,167
Bruce (1985)	Case 2	Olympia Harbor (Washington)	1,5714286	6	0,156	0,196	0,165	0,140	0,031	1,231
Torun (1987)	Case 3	Physical Model	1,4285714	4,6869141	0,123	0,223	0,091	0,088	0,027	1,401
Manuel (1995)	Case 4	Physical Model	1,3333333	2	0,032	0,620	0,248	0,200	0,020	3,895
Murali (1997)	Case 5	Physical Model	1,4347826	2,826087	0,200	0,108	0,141	0,460	0,022	0,680
Sannasiraj (1998)	Case 6	Physical and Numerical Model	4	4	0,019	0,510	0,087	0,043	0,010	3,203
Allyn (2004)	Case 7	Physical Model	1,2352941	2,2941176	0,076	0,326	0,127	0,170	0,025	2,046
Fouster (2007)	Case 8	Numerical Model	1,25	2,25	0,123	0,396	0,178	0,200	0,049	2,488
Martinelli (2008)	Case 9	Physical Model	2,3333333	6,6666667	0,170	0,246	0,105	0,064	0,042	1,545
Elchahal (2009)	Case 10	Numerical Model	1,1315789	0,6644737	0,050	0,401	0,101	0,380	0,020	2,518
Wang (2010)	Case 11	Numerical Model	1,1111111	1,6	0,150	0,168	0,135	0,450	0,025	0,731
Yoon (2011)	Case 12	Physical and Numerical Model	2,6666667	6,6666667	0,086	0,116	0,106	0,136	0,010	0,731
He (2012)	Case 13	Physical Model	2,259887	8,7570621	0,044	0,225	0,388	0,197	0,010	1,414
Loukogeorgaki (2012)	Case 14	Numerical Model	1,8181818	4,5454545	0,037	0,158	0,250	0,347	0,006	0,995

- FB has a position which is fixed in space and infinitely long in longshore direction
- Ideal, weakly compressible and irrotational flow are assumed
- Wave propagation perpendicular to the beach
- Model was validated running 160s , T= 8,7, and 6s and $X_R=50, 75, 100$ and 150 m

3. Methodology



- To validate the model 90 cases were executed in 2D and the behavior of 630 waves were observed.
- To select the proposed structure for Santa Marta 75 cases were executed and 525 waves evaluated .
- All simulations were executed using GPU (Graphics Processing Units) with support from the Environmental Physics Laboratory of the University of Vigo.

3. Methodology

a. FB Study Case Design

- If the Box model behavior is like closed box.... Resonance????
- Phenomenon that describes the processSeiches!!!!

“Long period oscillations that occur in closed or semi-closed basins, which to generate resonant wave height increases inside”

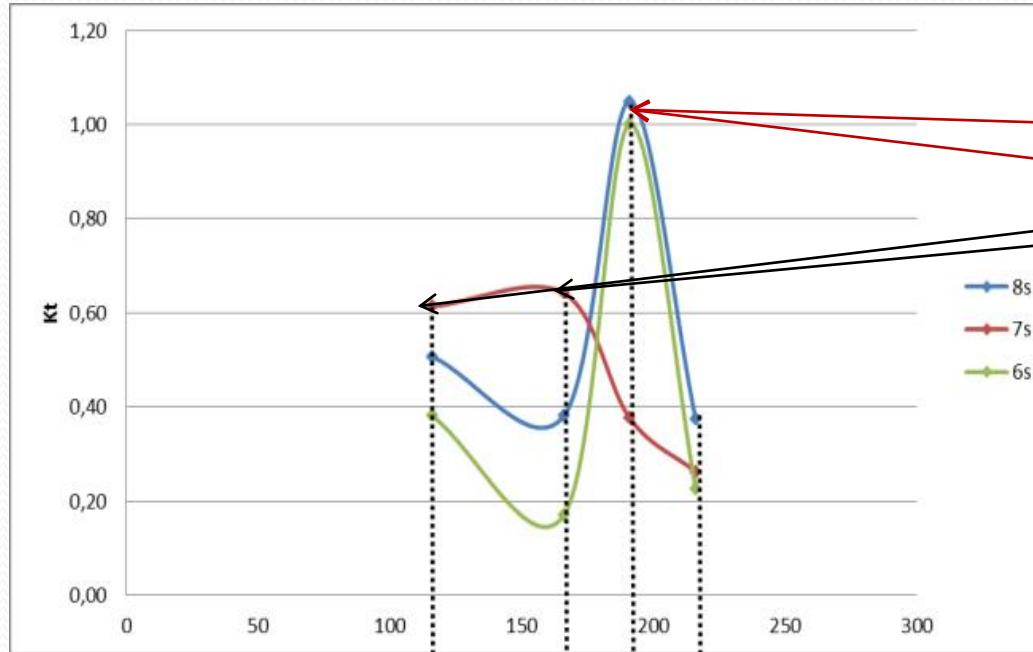
- *Merian (1828)*

$$T_n = \frac{2L}{n\sqrt{gh}} \quad (6)$$

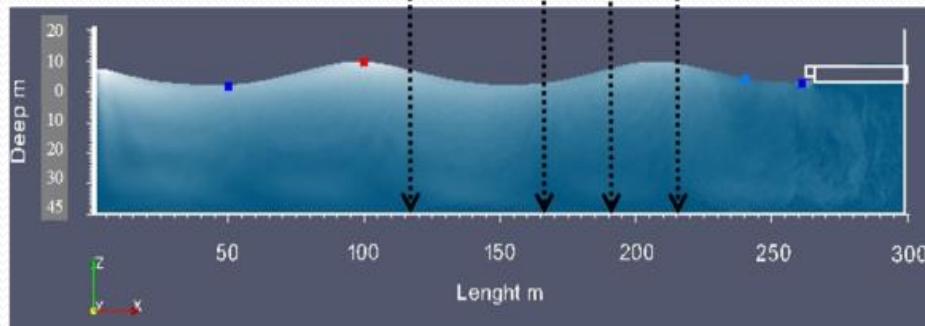
- where T_n is the natural period of oscillation, L is the length of the domain, n the number of node.
- relationship proposed by Sorensen (1993) wavelength λ and the domain length L to determined a critical ratio.

4. Results and Discussion

a. Box Model Validation



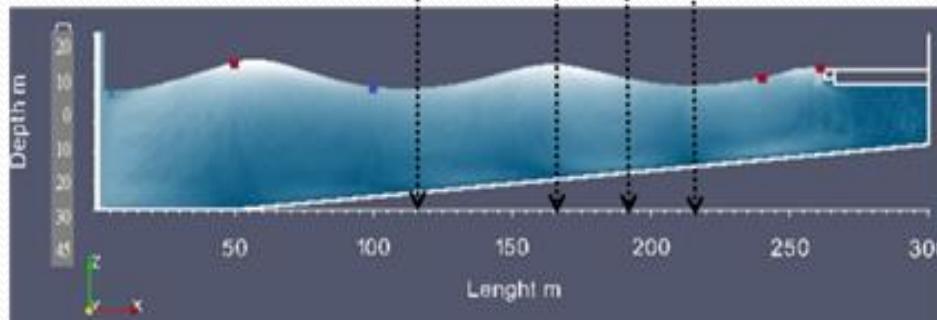
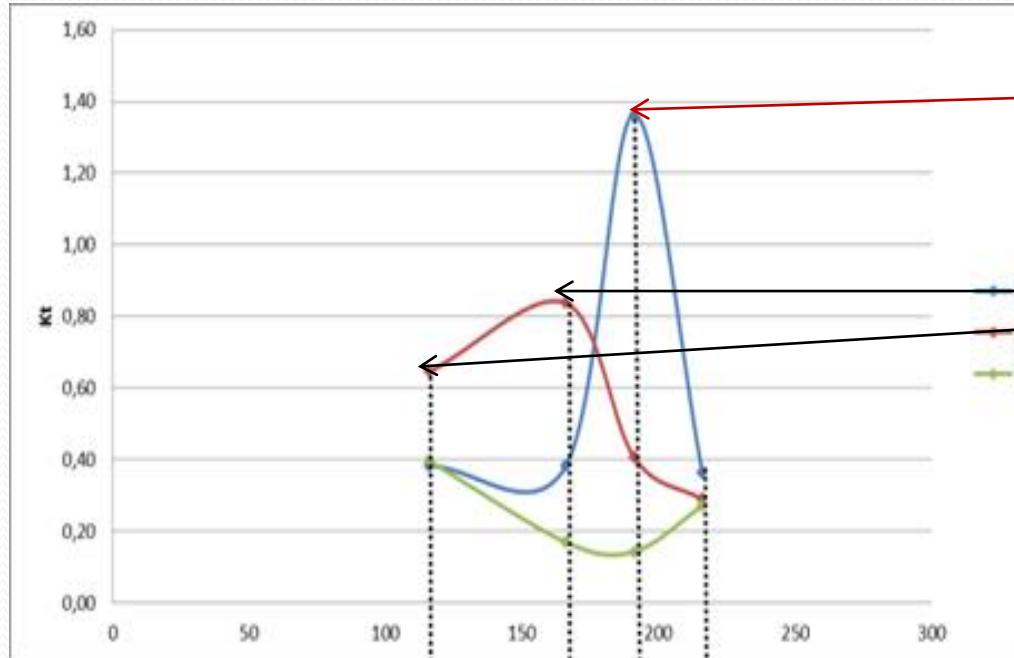
FB Depth	h	45	λ	λ/L			
				50	75	100	150
	8	99,576	1,9915	1,33	1	0,66	
T	7	76,457	1,5291	1,02	0,76	0,51	
	6	56,205	1,1241	0,75	0,56	0,37	



Open-ended Basin Critical Periods	0,25	0,75	1,25	1,75	2,25
Closed Basin Critical Periods	0,5	1	1,5	2	2,5

4. Results and Discussion

a. Box Model Validation

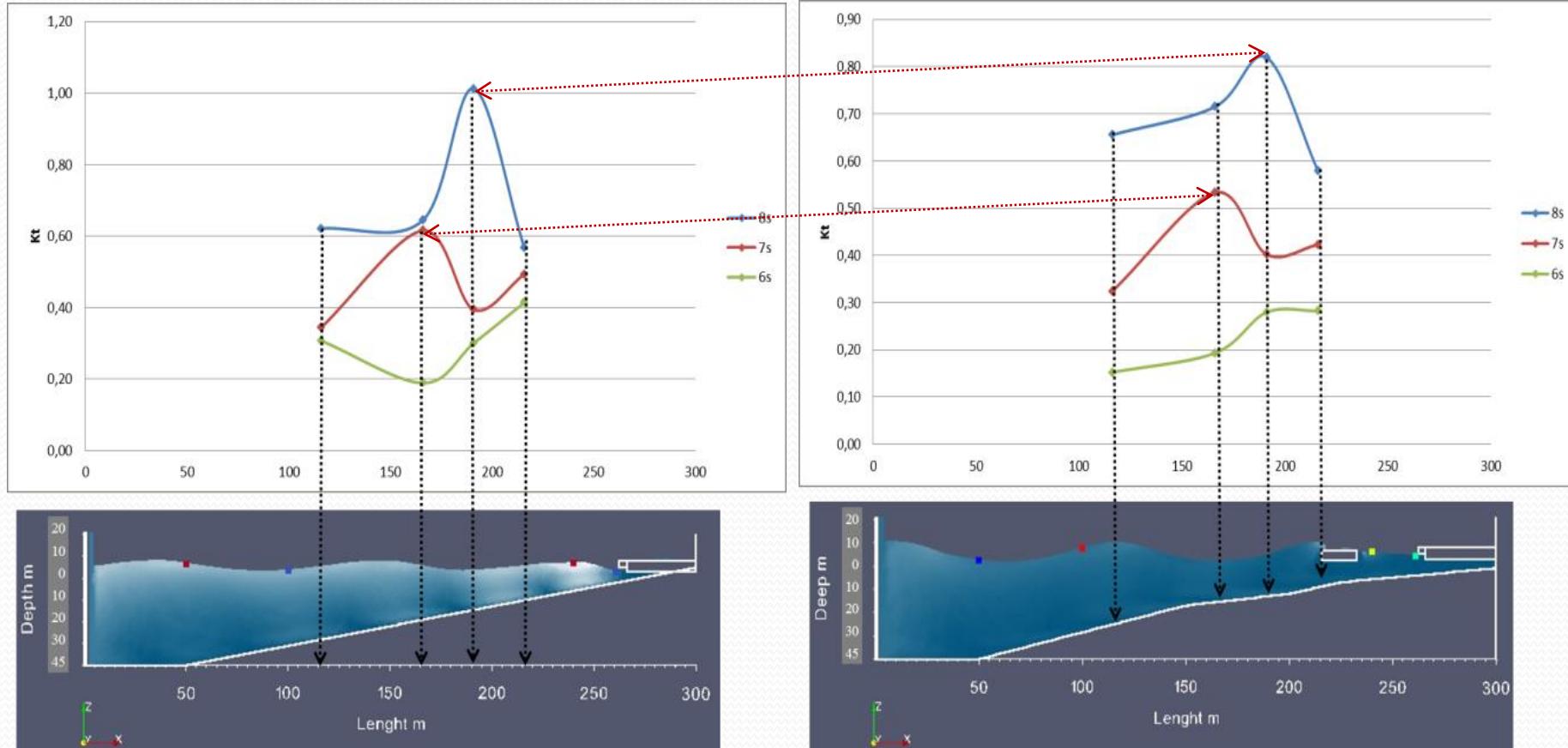


FB Depth		λ	λ/L	FB Depth		λ	λ/L
h	28	50	75	h	30	75	100
T	8	97,012	1,9402	T	8	97,653	1,302
	7	75,738	1,5148		7	75,952	1,0127
	6	56,1	1,122		6	56,139	0,7485
FB Depth		λ	λ/L	FB Depth		λ	λ/L
h	32	100	150	h	37	150	100
T	8	98,153	0,9815		8	98,976	0,6598
	7	76,105	0,7611		7	76,339	0,5089
	6	56,163	0,5616		6	56,193	0,3746

Open-ended Basin Critical Periods	0,25	0,75	1,25	1,75	2,25
Closed Basin Critical Periods	0,5	1	1,5	2	2,5

4. Results and Discussion

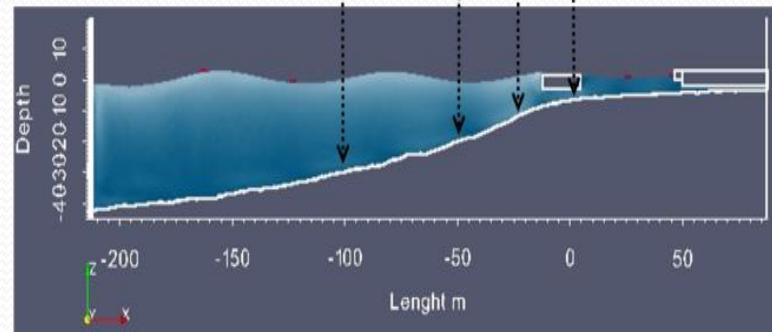
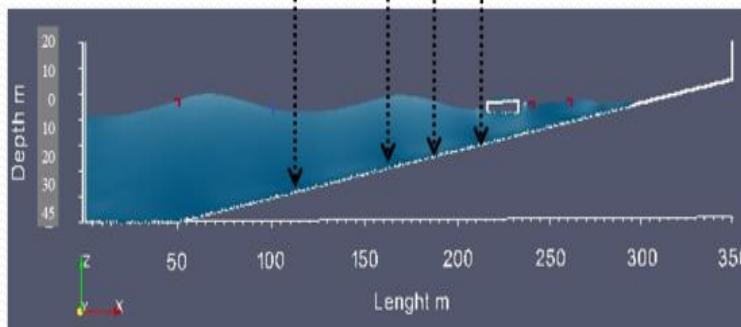
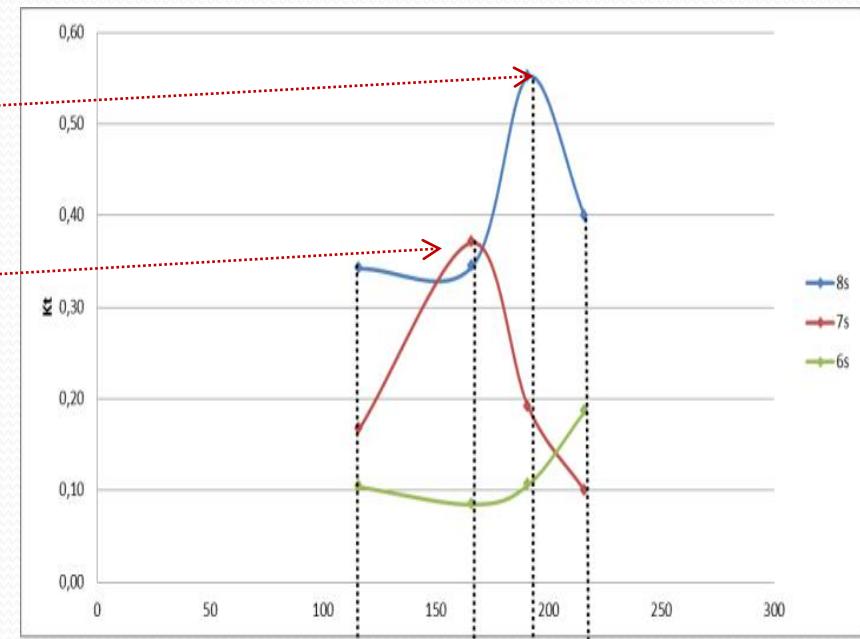
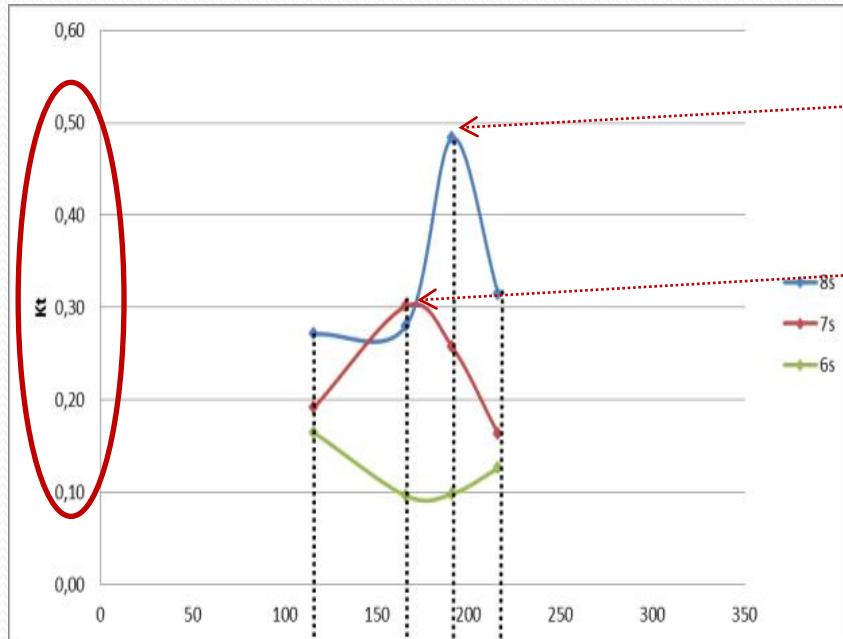
a. Box Model Validation



- If in all cases there + / - resonance We had fictitious results???

4. Results and Discussion

a. Box Model Validation



4. Results and Discussion

a. Box Model Validation

- Reflection and resonance are presented in 2D
- There is good correlation between the best condition simulated and the case study
- The box model behaves as closed and semi-closed basin depending on the distance between the FB and Pier
- Overall 2D model validation!!!!!!

4. Results and Discussion

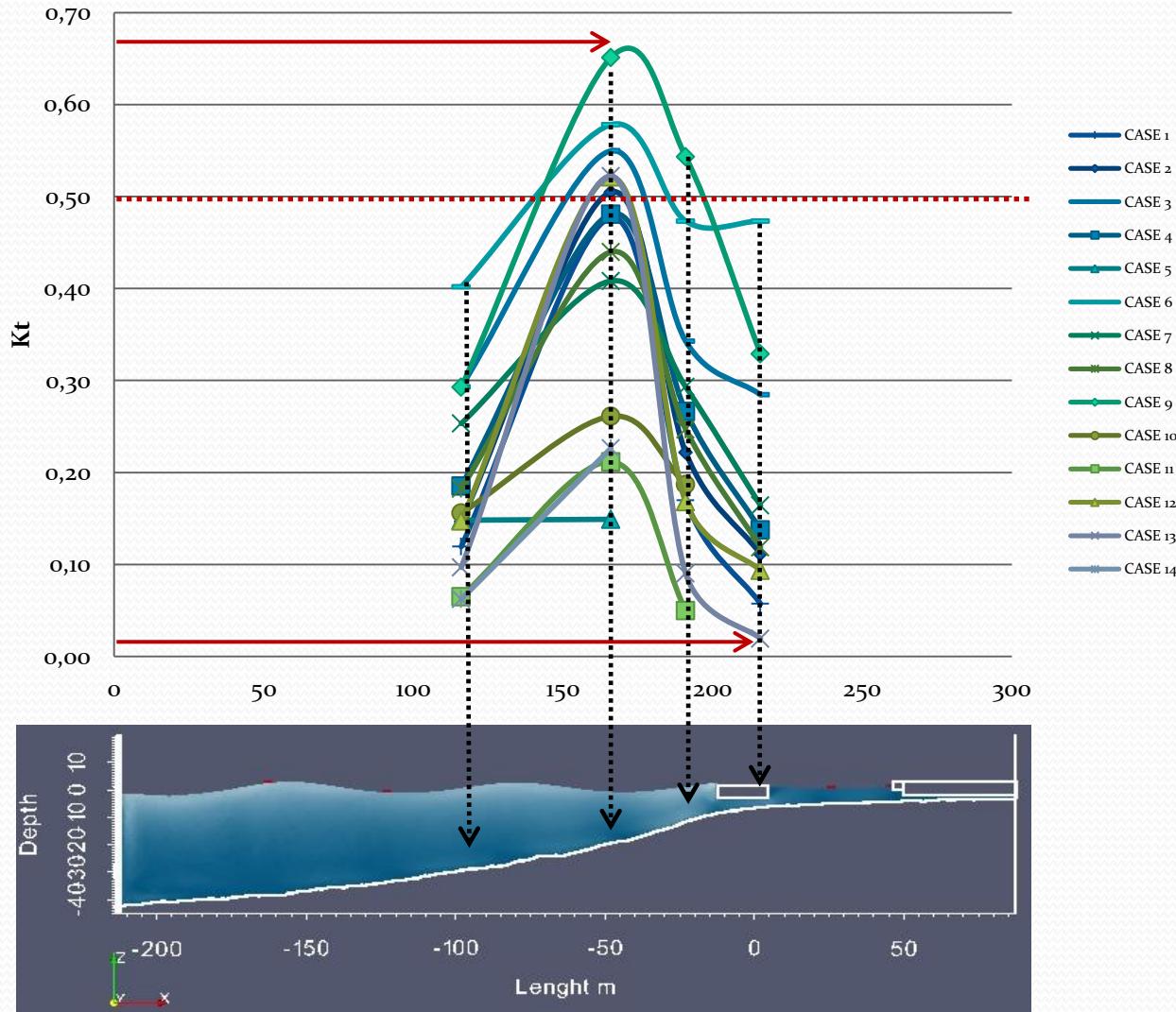
b. Santa Marta Case

- FB Reconstructions:

PARAMER/CASE	1	2	3	4	5	6	7	8	9	10	11	12	13	14
Z _R	6,55	4,4	2,5	5,33	13,2	3,4	4,2	5	2,97	8,6	10	7,27	8,88	12,63
X _R	22	16,8	8,2	8	26	3,4	7,8	9	8,51	5,05	14,4	18,18	34,4	31,57
Dr	2,93	2,8	1,75	4	9,2	0,85	3,4	4	1,27	7,6	9	2,72	3,93	6,94
ERROR	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25	(+/-) 0,25
Area FB m ²	144,1	73,92	20,5	42,64	343,2	11,56	32,76	45	25,27	43,43	144	132,2	305,5	398,7

4. Results and Discussion

b. Santa Marta Case



4. Results and Discussion

b. Santa Marta Case

Set 1	FB Vol.	Kt	Overall
1	0,60	0,40	1,00
11	0,8	0,8	1,60
12	1,00	0,4	1,40
13	0,40	1,00	1,40
14	0,2	0,60	0,80

Set 2	FB Vol.	Kt	Overall
2	0,33	1,00	1,33
4	0,83	0,66	1,49
5	0,16	0,5	0,66
7	1,00	0,33	1,33
8	0,50	0,83	1,33
10	0,66	0,33	0,99

Set 3	FB Vol.	Kt	Overall
3	0,66	1,00	1,66
6	1,00	0,33	1,33
9	0,33	0,66	0,99

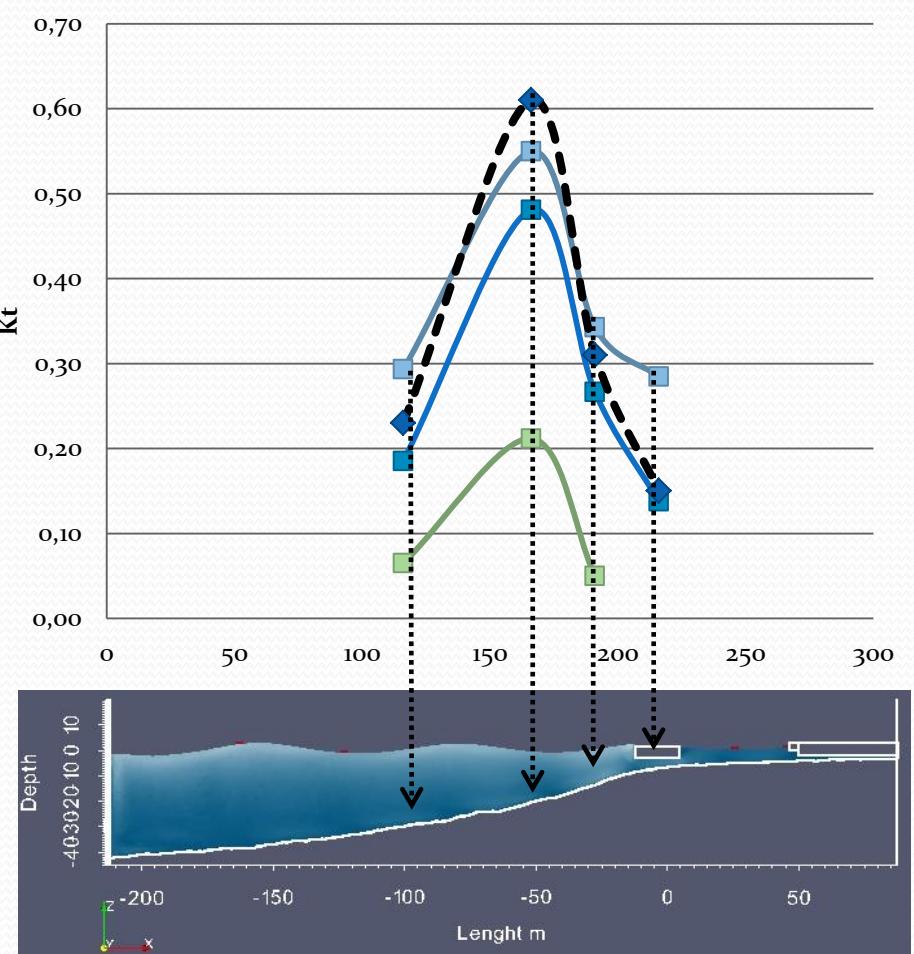
- Santa Marta Case 15:
 - ZR= 6.5m
 - XR= 6.0m
 - DR= 4.0m

Area: 39m^2

Volume 3D: 1560m^3

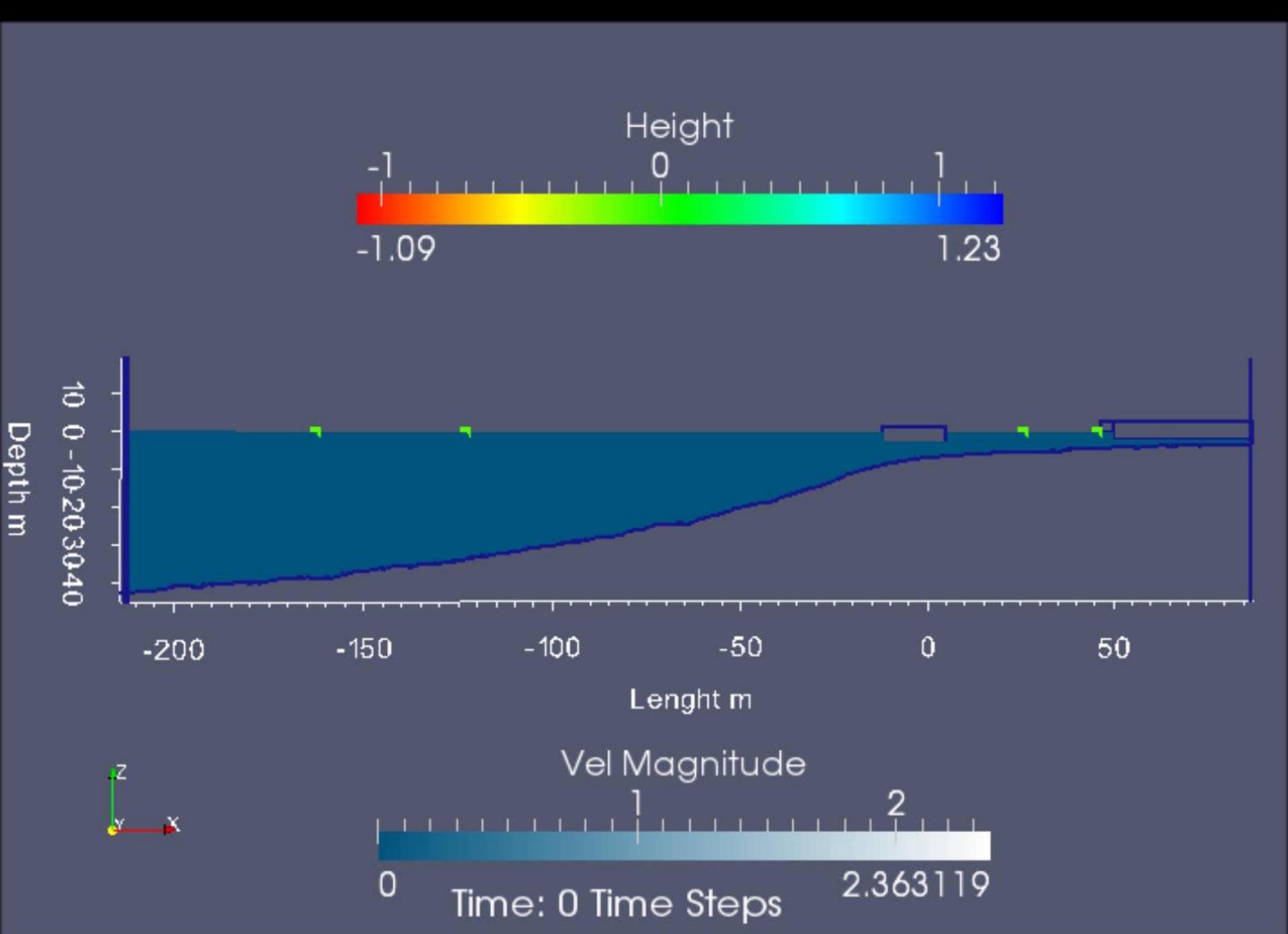
4. Results and Discussion

b. Santa Marta Case



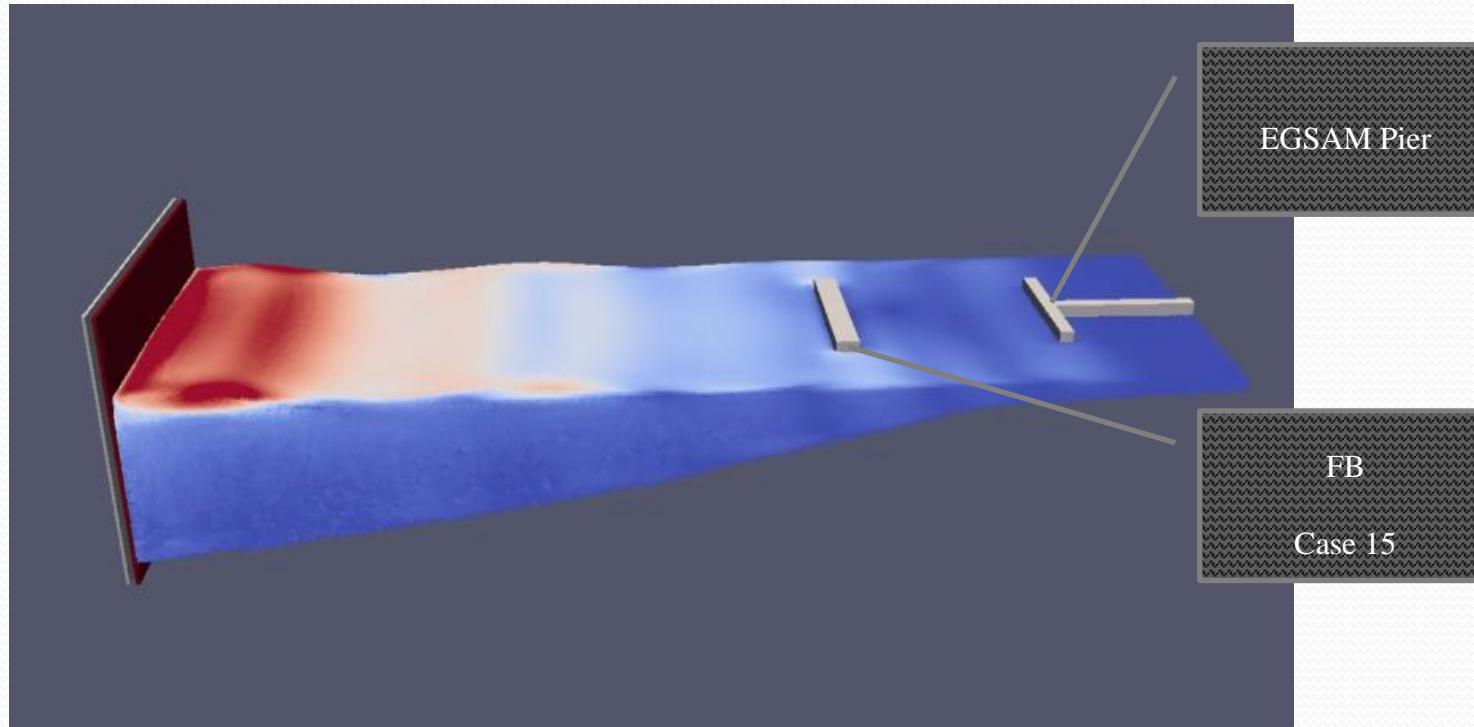
Case	Area m ²
3	20.5
4	42.6
11	144
15	39

- Despite having an area substantially smaller maintains adequate efficiency ranges.
- Now....3D Simulations.



4. Results and Discussion

b. Santa Marta 3D Case

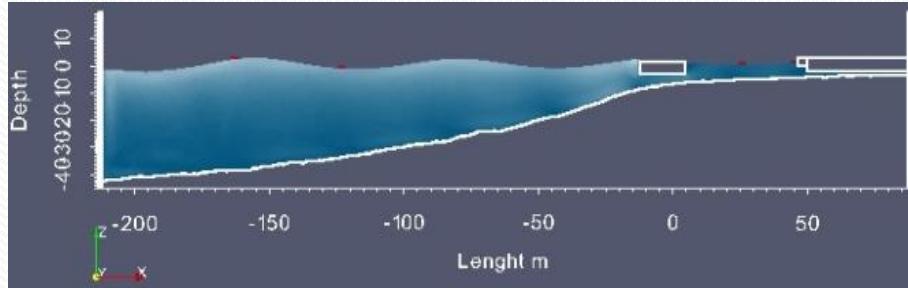


- Diffraction effects on the sides of the FB
- Reflection effect throughout the pier and the FB/Domain
- Energy dissipation by shoaling effect in the back zone to the pier
- Wave set was stabilized at 50 time steps vs. the 350 requiring in 2D

4. Results and Discussion

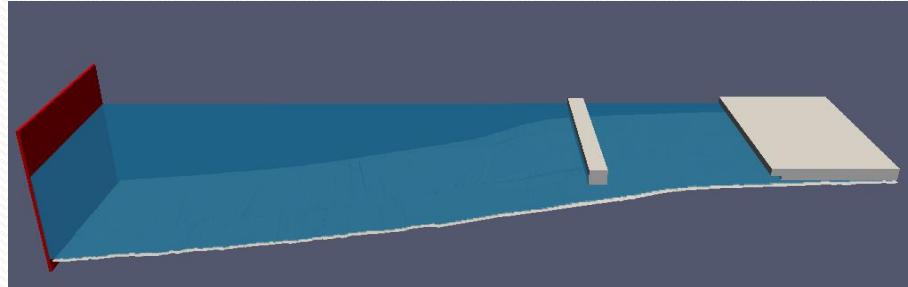
b. Santa Marta 3D Case

1°)



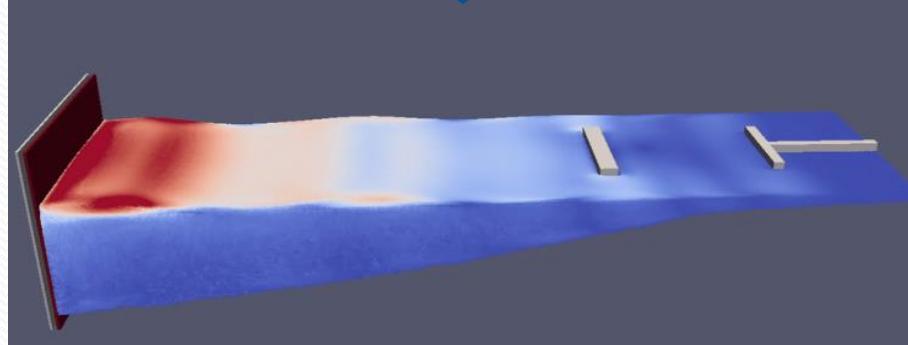
PROMISING RESULTS!
2D with resonance
2D without diffraction

2°)



EXTENSION TO 3D CASE
3D with resonance
3D without diffraction

3°)

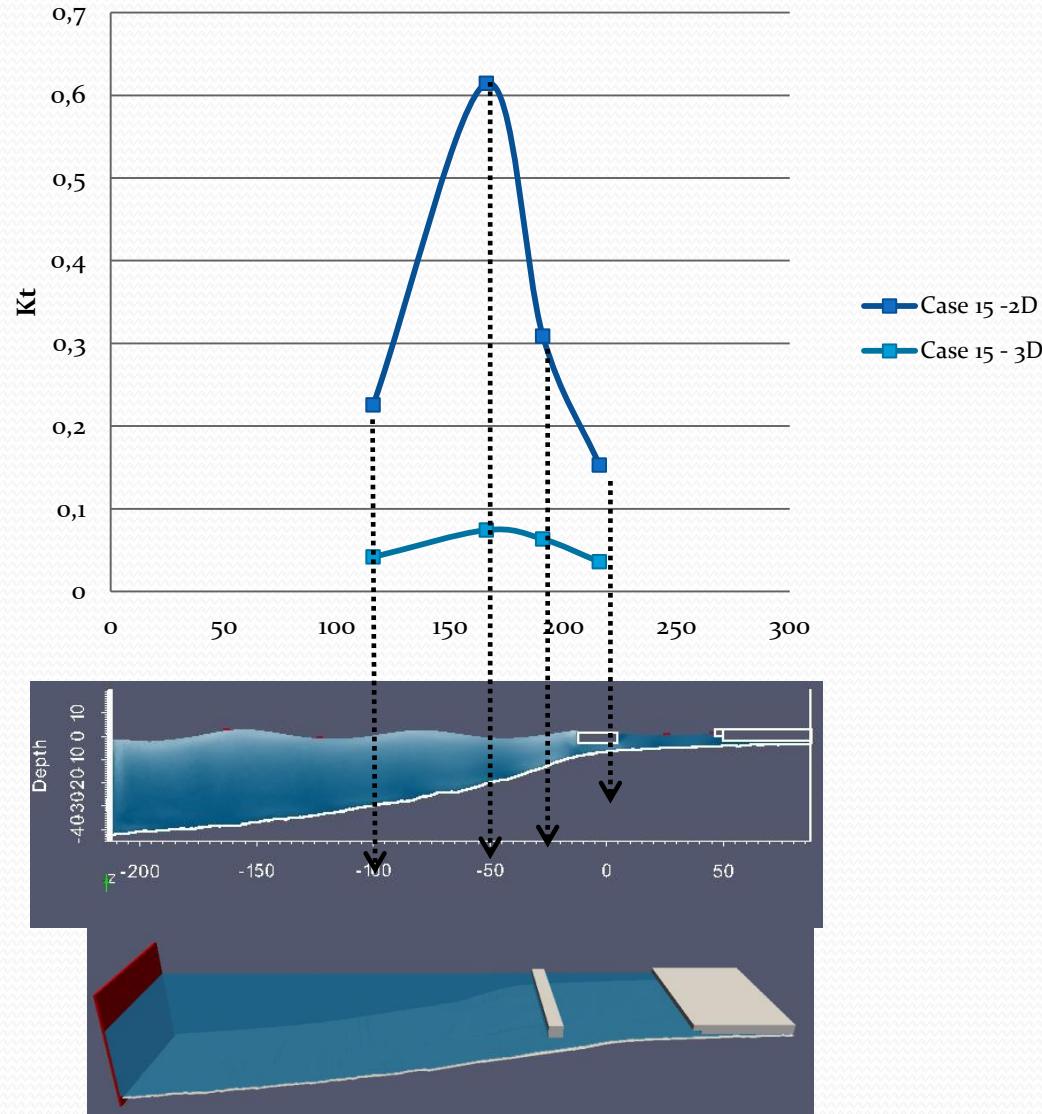


REALISTIC 3D CASE
3D without resonance (beach)
3D with diffraction

We must remove the effect of
lateral walls

4. Results and Discussion

b. Santa Marta 2D vs. 3D Case

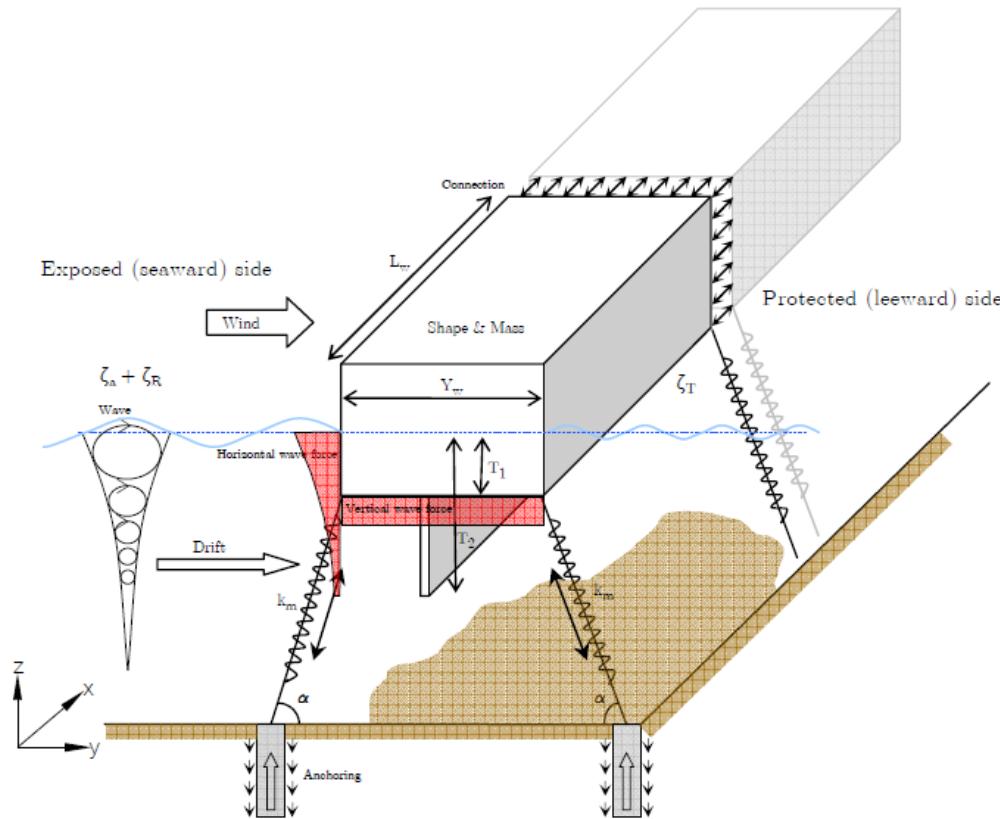


5. Conclusiones

- The FB efficiency, evaluated under real conditions, let's see use in open water areas, under the influence of waves with periods up to 7s.
- The SPH method is adequate to simulate with accuracy the interaction between waves and structure under the effect of an irregular bathymetry.
- Although in reality the conditions of reflection domain boundaries (3D) and resonance (2D) present in the model does not exist, the values remain high precision value.
- The proposed structure for the EGSAM pier has significant advantages and accuracy of data, which must be evaluated in a wave channel, as part of the design process.
- We show that a structure with an area of $39m^2$, or with a volume of $1560 m^3$ as those studied in this work, can protect a medium port works with a lower degree of environmental impact

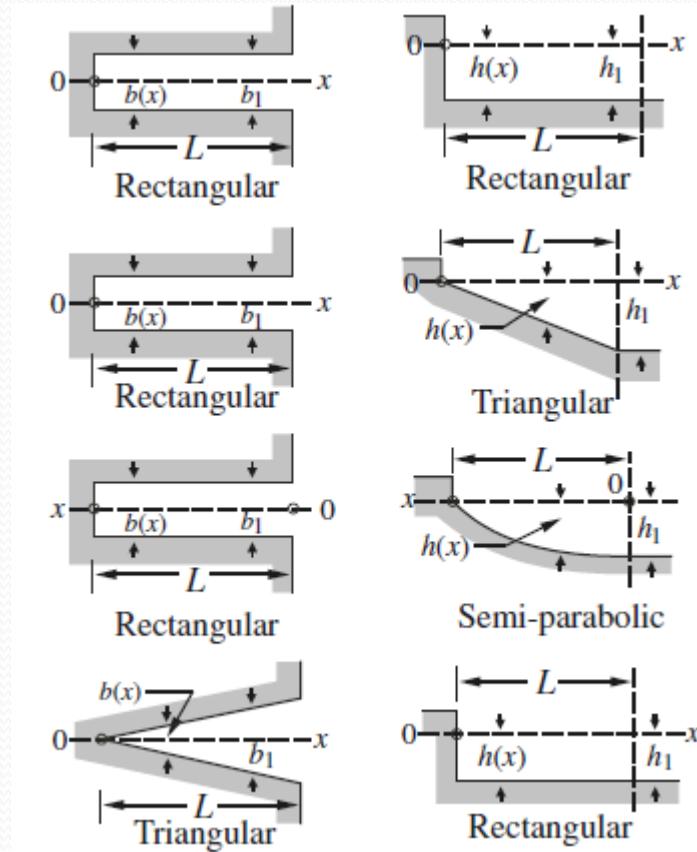
6. SPH Future Research

- Floating Breakwaters with free movement



6. SPH Future Research

- Resonance

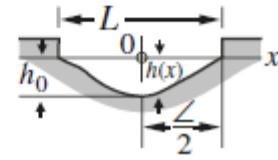
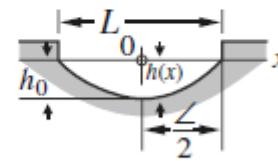
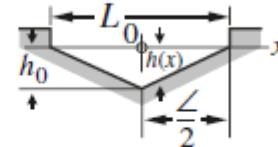
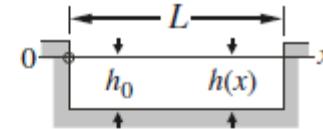


Rectangular

Triangular
(isosceles)

Parabolic

Quartic



References

- Adee, B., Floating Breakwater Performance. *Coastal Engineering*, 15, pp.2777-2791, 1976.
- Allyn, N., Watchorn, ernie., Jamieson, W., and Yang, G., Port of Brownsville Floating Breakwater. *Proc. Ports Conference*, 2001.
- Batchelor, G., *Introduction to Fluid Dynamics*. Cambridge University Press, 1974.
- Brebner, A., and Ofuya, A. Floating Breakwaters, Proceeding of 11th Conference on Coastal Engineering, London, 1968, ASCE, 2, pp. 1055-1094, 1969.
- Bruce, L., Floating Breakwater Design, *J. Waterway, Port, Coastal, Ocean Eng.* 111, pp. 304-318, 1985.
- Carr, J. H., Mobile Breakwater Studies: Hydrodynamics. Laboratory Report No. N-64.2, California Institute of Technology, 1950.
- Centro de Investigaciones Oceanográficos e Hidrográficos. *Climatología de los Principales Puertos del caribe Colombiano*. 2010.
- Chen, k., Wiegel, R.L., Floating breakwater for reservoir marines. *Proc. Of the 12th Coastal Engineer*. Pp. 487-506, 1970.
- Crespo, A.J.C., Gómez-Gesteira, M. and Dalrymple, R.A., 3D SPH simulation of large waves mitigation with a dike. *Journal of Hydraulic Research*, 45(5), pp. 631- 642, 2007a.
- Crespo, A.J.C., Gómez-Gesteira, M., Dalrymple, R.A., Modeling Dam Break Behavior over a Wet Bed by a SPH Technique. *Journal of Waterway, Port, Coastal, and Ocean Engineering*, 134(6), pp. 313-320, 2008.
- Dalrymple, R. A., Rogers, B., Numerical modeling of water waves with the SPH method. *Coastal Engineering*, 53, pp. 141-147, 2006.
- De la Sala, P., Revista de obras públicas. Rompeolas Flotantes. Vol. 14; pp. 165-167 and Vol.15, pp.178-181. 1873.
- Fousert, M., Floating Breakwater: Theoretical Study of a Dynamic wave Attenuating System. Master Thesis, Delft University of Technology, 2006.
- Elchahal, G., Lafon, P., and Younes, R., Design Optimization of Floating Breakwaters with an interdisciplinary Fluid-Solid Structural Problem. *Can. J. Civ. Eng.*, 36, pp. 1732-1743, 2009.
- Gingold, R., and Monaghan, J., Smoothed Particle Hydrodynamics: Theory and Application to Non-Spherical Stars. *Mon Not R Astr Soc*, 181, pp. 375-389, 1977.

References

- Gómez-Gesteira, M., Dalrymple, R.A., Using a three-dimensional smoothed particle hydrodynamics method for wave impact on a tall structure. *Journal of Waterway, Port, Coastal and Ocean Engineering*, 130(2), pp. 63–69, 2004.
- Gómez-Gesteira, M., D. Cerqueiro., A. Crespo., and R. Dalrymple., Green water overtopping analyzed with a SPH model. *Ocean Engineering*, 32, pp. 223-238, 2005.
- Hales, Z.L., Floating Breakwaters: State of the Art, U.S Army Corps of Engineers. Technical Report No 81-1 Cap. 1. pp. 23-45, 1981.
- He, F., Huang, Z., Wing, A., Hydrodynamic Performance of a rectangular Floating Breakwater with and without Pneumatic Chambers: An Experimental Study. *Ocean Engineering*, 51, pp. 16-27, 2012.
- Isaacson, M., and Sinha, s., Directional Wave effects on Large Offshore Structures. *Journal of Waterway, Port, Coastal and Ocean Engineering*. 112 (4), pp. 482-497, 1986.
- Isaacson, M., and Nwogu, O.U., Wave Loads and Motions of Long Structures in Directional Seas. *Journal of Offshore Mechanics and arctic Engineering*, 109, pp. 126-132, 1987.
- Jones, D., Transportable Breakwaters—A Survey of Concepts, Technical Report R-727, U.S. Navy Civil Engineering Laboratory, 1971.
- Koutandos, E., Karambas, Th., Koutitas, C., Prinos, P., Floating Breakwaters Efficiency in Intermediate and Shallow Waters. Procedding of the 5th International Conference on Hydro -Science and Engineering, Portland, pp. 1-10, 2002.
- Koutandos, E., Prinos, P., Gironella, X., Floating Breakwaters under Regular and Irregular Wave Forcing: Reflection and Transmission Characteristics. *Journal of Hydraulic Research*, 43(2), pp. 174-188, 2005.
- Lucy, I., A numerical Approach to the testing of the Fission Hypothesis. *Astron. J.*, 82(12), pp. 1013-1024, 1977.
- Lo, E., And Shao, S., Simulations of Near-Shore solitary Wave Mechanics by an incompressible SPH Method. *Applied Ocean research*, 24, pp. 275-286.
- Loukogeorgaki, E., Michailides, C., and Angelides, C., Hydroelastic Analysis of a Flexible Mat-Shaped Floating Breakwater under Oblique wave Action. *Journal of Fluids and structures*. 31, pp. 103-124, 2012.
- Manuel, B., Response of a Pile restrained Floating Breakwater. Master Thesis, University of British Columbia, 1997.
- Martinelli, L., Piero, R., Zanuttigh, B., Wave basin experiments on floating breakwaters with different layouts. *Applied Ocean research*, 30, pp. 199-207, 2008.
- Monaghan, J., Smoothed Particle Hydrodynamics. *Annual Rev Astron. Appl.*, 30, pp. 543-574, 1992.

References

- Monaghan, J., Simulating free Surface Flows with SPH. *Journal Computational Physics*, 110, pp. 399-406, 1994.
- Morris, J., and Kos, A., Solitary Waves on a Cretan Beach. *Journal of Computational Physics*, 136, pp. 214-226, 1997.
- Murali, K., Amer, S. S., and Mani, J. S. Dynamics of cage floating breakwater. *Journal of Offshore Mechanics and Arctic Engineering. Transactions of the ASME*, 127(4), pp. 331-339, 1997.
- Najafi, A., and Mazyak, A., Numerical Investigation of Floating Breakwater Movement using SPH Method. *Inter J Nav Oc Engng*, 3, pp. 122-125, 2011.
- Peña, E., Ferreras, J., and Sanchez-Tembleque, F., Experimental study on wave transmission coefficient, mooring lines and module connector forces with different designs of floating breakwaters. *Ocean Engineering*, 38, pp.1150-1160, 2011.
- Permanent International Association of navigation Congress – PIANC, *Floating Breakwaters: A Practical Guide for Design and Construction*. Report of working group No 13 of the permanent technical committee II, 1994.
- Ramos, L., Vidal, L., Vilardy, S., Saavedra. L., Analysis of The Microbiological Contamination (Total And Fecales Coliforms) In The Bay Of Santa Marta, Colombian Caribbean. *Acta biol. Colombiana*, Vol. 13 (3) pp. 87 - 98, 2008.
- Rabinovich, A., Long Ocean Gravity waves: Trapping, Resonance, and Leaking. *Gidrometeoizdat*, (in Russian), pp. 325, 1993.
- Richey, E.P., and Nece, R.E. , "Floating Breakwaters: State-of-the-Art," *Proceedings of the Floating Breakwaters Conference*, University of Rhode Island, Kingston, R.I., pp. 1-20. (1974).
- Sannasiraj, SA., Sundar, V., Sundaravadivelu, R., Mooring Forces and Motion Responses of Pontoon-Type Floating breakwaters. *Ocean Engineering*, 25(1), pp. 27-48, 1998.
- Sannasiraj, SA., Sundaravadivelu, R., Sundar, V., Difraction-radiation of Multiple Floating Structures in Directional Waves. *Ocean Engineering*, 28(2), pp. 201-234, 2000.
- Shao, S., SPH simulation of a solitary wave interaction with a curtain-type breakwater. *Journal of Hydraulic research*, 43 (4), pp. 366-375, 2005.
- Sorensen, R., *Basic Wave Mechanics: for Coastal and ocean Engineers*. John Wiley & Sons, inc. pp.241, 1993.
- Stiassnie, M., A simple mathematical model of a floating breakwater, *Applied Ocean Research*, 2 (3), pp. 107-111, 1980.



- Questions....

Acknowledgements

- A mi esposa!!!!!!! Y mi Familia....
- Armada República de Colombia
- Fundación Carolina
- Universidad de las Palmas de Gran Canaria.....
 - PhD Germán Rodriguez
 - Profesores y compañeros de clase...
- Environmental Physics Laboratory de la Universidad de Vigo
- PhD Alex Crespo
- Jose Dominguez
- Centro de Investigaciones Oceanográficas e Hidrográficas – PhD Sergei Lonin